SOIL COMPACTION INVESTIGATION

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EVALUATION OF VIBRATORY ROLLERS ON THREE TYPES OF SOILS

J. W. Hall



March 1968

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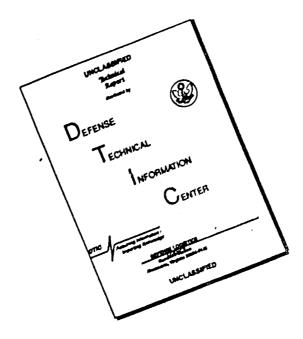
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SOIL COMPACTION INVESTIGATION: REPORT 10. EVALUATION OF VIBRATORY ROLLERS ON THREE TYPES OF SOILS

Jim W. Hall

Army Engineer Waterways Experiment Station Vicksburg, Mississippi

March 1968

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SOIL COMPACTION INVESTIGATION

Report 10,

EVALUATION OF VIBRATORY ROLLERS ON THREE TYPES OF SOILS

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FOREWORD

This investigation is part of the continuing Air Force program sponsored through Military Construction, Office, Chief of Engineers, for developing construction methods and techniques for flexible pavements.

The study described herein was accomplished by personnel of the Soils Division, U. S. Army Engineer Waterways Experiment Station, during the period May through June 1962. Engineers actively engaged in the collection and analysis of data were Messrs. C. D. Burns, A. H. Joseph, and J. W. Hall. The overall study was under the general supervision of Messrs. W. J. Turnbull, A. A. Maxwell, and R. G. Ahlvin.

Director of the Waterways Experiment Station during the conduct of this study was COL John R. Oswalt, Jr., CE. Technical Director was Mr. J. B. Tiffany.

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CONVERSION FACTORS, EXITISH TO METRIC UNITS OF MEASUREMENT

British units of measurement used in this report can be converted to metric units as follows:

Multiply	Ву	To Obtain
inches	2.54	centimeters
feet	0.3048	meters
pounds	0.45359237	kilograms
pounds per square inch	0.070307	kilograms per square centimeter
pounds per cubic foot	16.0185	kilograms per cubic meter
tons	907.185	kilograms

SUMMARY

This study was conducted for the purpose of determining the ability of vibratory rollers to compact soils. For comparative purposes, a 50-ton rubber-tired roller was used which is a required compaction device in present Corps of Engineers Guide Specifications.

Three vibratory rollers were selected for study based on their operating frequency which encompassed the range over which present vibratory rollers operate. Results of this study show that light vibratory rollers can obtain satisfactory densities if lift thicknesses are restricted. To evaluate the vibratory rollers, each was used to compact three soil types (a lean clay, a crushed limestone, and a clean sand).

Results indicate that a heavy, low-frequency vibratory roller will compact to greater depths than a light, high-frequency roller; however, the light, high-frequency roller will compact soil satisfactorily for a few inches below the surface.

Soil types have a very definite influence on results obtained with vibratory rollers. The vibratory rollers generally perform better in granular soils; however, the heavy, low-frequency type rollers do a satisfactory job in clay soils.

SOIL COMPACTION INVESTIGATION

EVALUATION OF VIBRATORY ROLLERS ON THREE TYPES OF SOILS

PART I: INTRODUCTION

Background

1. The appearance in the past decade of a large variety of vibratory equipment for the compaction of soils has given rise to the question of whether lightweight vibratory equipment can be substituted for conventional compaction equipment, which generally depends on its deadweight to densify soils. Most equipment manufacturers that build compaction equipment are now producing some type of vibratory machine for compacting soils. A large percentage of the vibratory equipment being produced makes use of the principle of an eccentric weight on a rotating shaft to produce the vibrations. The eccentrically weighted rotating shaft is connected to a drum or a plate that is used to transfer the vibrations to the soil. The manufacturers' rating of these compactors is based on the force generated by the eccentrically weighted rotating shaft. This system of rating the compactors is misleading because it ignores the phase lag between the point of generation at the eccentric and the point of compaction within the soil. This rating system is based on the centrifugal force equation:

 $F = Me\omega^2$

where

F = force, lb*

M = mass of the eccentric weight, $\frac{\text{weight}}{\text{gravity}} = \frac{1b}{\text{ft/sec}^2}$

e = distance from center of gravity of eccentric mass to center of rotation, in.

 ω = angular velocity of the rotating mass in radians per minute, and is equal to (2π) (rpm)

This equation gives the centrifugal force generated at the center of

^{*} A table of factors for converting British units of measurement to metric units is presented on page vii.

rotation of the eccentric, but due to damping in the roller system and to the relation of the inertial forces of the roller to the generated forces, it is difficult, if not impossible, to compute the compaction effort received by the soil. Flexibility within the roller, but more particularly in the soil, produces damping which can reduce the generated force to a fraction of its original magnitude.

Purpose and Scope

The purpose of this investigation was to determine the effectiveness of vibratory rollers for compacting soils. To accomplish this purpose, three vibratory rollers were obtained for the study. The selection of the vibratory rollers was based primarily on the frequency at which they operated with respect to the frequency range over which most vibratory rollers operate. One each from the low-, medium-, and highfrequency ranges was selected for testing. For comparison purposes, a 50-ton rubber-tired roller was also used. In order that the rollers could be evaluated over a range of conditions, each reller was used to compact three types of soils: a crushed limestone, a fine sand, and a lean clay. For each soil, the moisture content was controlled at three different levels, i.e. at wet of optimum, optimum, and dry of optimum, based on the optimum moisture content as obtained from the modified AASHO (MIL-STD-621, CE55) moisture-density relation. (Hereinafter, MIL-STD-621, CE55 moisture-density relation will be referred to simply as modified AASHO maximum density.) This variation of moisture content produced nine conditions of compaction for each roller operated (i.e. three soil types and three moisture contents for each soil). The density of the soil and the depth to which compaction was being obtained were measured throughout the period of traffic by the rollers.

PART II: FIELD TESTS

Test Area

3. The field tests were conducted under shelter at the Waterways Experiment Station (WES) in order that better control could be maintained over all phases of the testing program. Four 90- by 10-ft test lanes were excavated for use in this compaction study, one lane for each of the vibratory rollers (lanes 1, 2, and 3) and a lane (lane 4) for the 50-ton rubber-tired roller. Each lane was divided into three items, which contained soil with dry of optimum, optimum, and wet of optimum moisture contents, respectively. Plate 1 shows a layout of the test section. The test procedure, which was as far as possible identical for each roller, consisted of the compaction of 1-ft-thick lifts of limestone and lean clay and 4-ft-thick lifts of sand (measurements are of compacted thicknesses). After the completion of tests on a particular soil, the soil was removed from the test trenches and replaced by another soil type and the compaction test was repeated. The lean clay material was mixed to the proper moisture content for each item before it was placed in the test trench. The limestone and sand were placed in the trenches with items separated by cutoff walls before the desired moisture was added and mixed into the materials.

Description of Rollers and Traffic Sequence

4. The physical characteristics of the vibratory rollers and the rubber-tired roller used in this compaction study are tabulated below:

Lane	Roller	Type of Roller	Mobility	Dead- weight	Drum Dimensions in.	rpm Kange	Centrifugal Force Generated at Max rpm, 1b
1	A	Vibratory	Towed	7,000	46×60	500 -16 00	15,000
2	В	Vibratory	Towed	3,150	30 × 54	3600	11,000
3	C	Vibratory	Self- propelled	,2 70	÷⊱ × €4	600-1400	25 , XCC
l _‡	D	Nubber- tired	Towed	100,000	(% tires, 1000x2% in., 20 psi)		

The sequence of traffic for compacting the soil in place had to be varied due to the difference in width of the rollers. For rollers A and B, two passes were required to make a complete coverage of the 10-ft-wide lane. The two passes overlapped 4 to 6 in. in the center of the lane to ensure complete coverage. Roller C, which is 84 in. wide, was passed over the center of the lane one time for a coverage. Two passes of the rubber-tired roller were required for a coverage. For each test lane, 16 coverages were applied.

Soils

5. The soils used in this compaction study (a crushed limestone, a sand, and a lean clay) were selected in an attempt to cover the range of materials generally used in pavement construction. Laboratory tests on these materials were conducted to determine the gradation and modified AASHO maximum density. Plate 2 is a plot of the gradation of the three soils used in this study. Plate 3 shows the moisture-density relations for the soils.

Tests Conducted

6. Since compaction was accomplished for only one lift thickness, it was not practical to determine the density gradient with depth after each coverage of the roller; however, surface densities were measured after 4, 8, and 16 coverages of each roller, and a complete density-depth relation was established after 16 coverages. The density values obtained at various depths at 16 coverages are shown for each soil type at dry of optimum moisture content, at optimum, and at wet of optimum for each of the rollers used in this study in tables 1, 2, and 3 for the sand, limestone, and lean clay, respectively. It can be noted from these tables that the after-traffic moisture contents for the sand and limestone were quite different from the moisture contents at which the soils were compacted. This is the result of the water draining from the high-porosity soils. There is a problem of maintaining moisture in granular soils during

compaction, and it is not known how these changes in moisture affected the results. During the first stages of compaction of the deep sand test lane with the rubber-tired roller, the roller would bog down and it was necessary to use an additional tractor to pull the roller through the sand. After several passes the rubber-tired roller could negotiate the sand lane without the additional tractor, but compaction did not occur from the surface down as it did in the case of the vibratory rollers. Comparisons will be made later herein between the vibratory rollers and the rubber-tired roller since the densities obtained with the rubber-tired roller are considered representative of its capability.

- 7. Each vibratory roller was instrumented to measure the frequency of its vibration during the compaction process. Instrumentation was also placed in each item of the soil being compacted to measure the frequency of vibration and the vertical movement of the soil. Velocity-type pickups were installed at the center position in each item at the 1-ft depth in the limestone, the lean clay, and lane 1 of the sand section, and at depths of 1, 2, and 4 ft in the items with optimum moisture contents in lanes 2 and 3 of the sand section. Table 5 summarizes the vertical movement of the soils measured by the ground instrumentation.
- 8. During the field testing in this study, a motorized automatic-recording nuclear moisture and density device (road logger) was used to check the moisture and density of the surface of the compacted soil behind each roller at 1, 2, 4, and 8 coverages. Portions of the data obtained with this device have been incorporated into this report to supplement the conventional data where needed; however, the primary purpose of the testing with this device was to evaluate the device itself. A report of this phase of the field testing will be published separately.

PART III: TEST RESULTS

Compactor A

Sand test section

9. Compactor A was a towed-type vibratory compactor with a gross weight of 7000 lb. The density-depth data taken after 16 coverages on the sand section are presented for all three moisture contents in table 1 and plate 4. The greatest compaction obtained by compactor A for each of the three moisture conditions was 92.2% of modified AASHO maximum density at the 18- to 24-in. depth in the dry of optimum moisture item, 96.2% of modified AASHO maximum at the 12- to 18-in. depth for the optimum moisture item, and 96.7% of modified AASHO maximum at the 6. to 12-in. depth for the wet of optimum moisture item. At greater depths than those corresponding to these maximum values, density fell off sharply as depth increased. Moisture was a big factor in the amount of densification in the sand section; the highest densities were obtained at the wet of optimum moisture content. It is interesting to note from table 4 and plate 7 that the densities in all three items decreased between 4 and 8 coverages and then increased between 8 and 16 coverages. The densities at 16 coverages were less than 1 pcf higher than those at 4 coverages for the optimum and wet of optimum moisture items, and about 7 pcf lower for the dry of optimum item. Overall, compactor A did the poorest job of any of the vibratory compactors in the sand--the densities obtained with compactor A and the rubber-tired roller were about the same at the surface; however, at the 12- to 18-in. depth compactor A obtained only about 90% of that obtained with the rubber-tired roller.

Limestone test section

10. The 16-coverage depth-density data for compactor A in the limestone section are given in table 2, and shown graphically in plate 5. The highest density obtained in the dry of optimum item was 92.5% of modified AASHO maximum at the 0- to 6-in. depth, the highest density in the optimum moisture was 98.1% of modified AASHO maximum at the 0- to 6-in. depth, and the highest density in the wet of optimum item was 97.9%

of modified AASHO maximum at the 6- to 12-in. depth. The highest density obtained in the limestone section by compactor A was the 98.1% of modified AASHO maximum at the O- to 6-in. depth for the optimum moisture item, but density in this item fell off with depth to a value of 95.8% at the 6- to 12-in. depth. Density values for the other two moisture conditions were practically constant for the entire depth of the section. The density-versus-coverage data given in table 4 and plate 8 show densities increasing with coverages in all three items. The increase in density between 4 and 16 coverages was 6.9 pcf at dry of optimum moisture and 5.2 pcf for both optimum and wet of optimum moisture conditions. The data used in obtaining plate 8 were the road logger data shown in table 4, and do not exactly agree with the values obtained by the direct sampling method which are also shown in table 4 for 16 coverages. The road logger data were the only data obtained at different coverage levels in the limestone section. Compactor A produced about 102% of the compaction developed by the rubber-tired roller for the full 1-ft depth in the wet of optimum item. Density values produced by compactor A for the other two moisture conditions were about 99.5% of those of the rubber-tired roller at the 0- to 6-in. depth and about 101% of those of the rubbertired roller at the 6- to 12-in. depth for both optimum and dry of optimum items.

Lean clay test section

11. The 16-coverage density-depth data for the lean clay section are presented in table 3 and plate 6. Compactor A obtained its highest density in the lean clay at the optimum moisture content. The maximum values for each item were 91.3% of modified AASHO maximum at the 0- to 2-in. depth for dry of optimum moisture, 94.1% of modified AASHO maximum at the 0- to 2-in. depth for optimum moisture, and 90.7% of modified AASHO maximum at the 0- to 2-in. depth for wet of optimum moisture. Compaction by compactor A was generally equivalent to that of the rubber-tired roller to a depth of 5 in.; as depth increased below that depth, compactor A densities dropped to a low of about 94% of those of the rubber-tired roller at the 8- to 10-in. depth. Density data were obtained at 4 and 16 coverages only and are given in table 4 and plate 9.

Compactor A densities increased between 4 and 16 coverages by 5.0 pcf at dry of optimum moisture content, 8.4 pcf at optimum moisture, and 3.3 pcf at wet of optimum moisture. The rubber-tired roller showed very little added densification between 4 and 16 coverages.

Compactor B

Sand test section

12. Compactor B was a towed-type vibratory compactor with a gross weight of 3150 lb. The 16-coverage density-depth data presented in table 1 and plate 4 show maximum densities to be 92.1% of modified AASHO maximum at the 6- to 12-in. depth for the dry of optimum moisture item, and 104.0% of modified AASHO maximum at the 6- to 12-in. depth for the wet of optimum moisture item. Plate 4 shows that the density increased with depth to a maximum of about 9 in. and decreased sharply below this depth. mum densification at each depth occurred in the wet of optimum moisture item. For the 0- to 12-in. depth, compactor B produced about 96% of the compaction produced by the rubber-tired roller at dry of optimum moisture, about 100% at optimum moisture, and 107% at wet of optimum moisture. Plate 4 shows that for depths greater than 9 in., the compactor B density values were all lower than those of the rubber-tired roller. The densitycoverage data presented in plate 7 show a decrease in dersity for compactor B between 4 and 8 coverages and an increase between 8 and 16 coverages to a maximum value for all three moisture contents.

Limestone test section

13. Maximum density values as given in table 2 for compactor B in the limestone section were 91.6% of modified AASHO maximum at the 0- to 6-in. depth for dry of optimum moisture, 94.7% of modified AASHO maximum at the 0- to 6-in. depth for optimum moisture, and 96.1% of modified AASHO maximum at the 0- to 6-in. depth for wet of optimum moisture. Therefore, the greatest density was obtained at wet of optimum moisture content. The rubber-tired roller produced its maximum density at the optimum moisture content. Plate 5 shows density values for compactor B to be much lower than those of the rubber-tired roller except at the 0- to

6-in. depth in the wet of optimum item where the values are about the same. The road logger density-coverage data presented in table 4 and plate 8 show practically no increase in density between 4 and 16 coverages for all three moisture contents, and a slight decrease in density between 4 and 8 coverages for the dry of optimum and wet of optimum conditions. Lean clay test section

14. The maximum density obtained in the lean clay section was 91.6% of modified AASHO maximum at the O- to 2-in. depth for optimum moisture content. Maximum densities in the dry of optimum and wet of optimum moisture items were 86.3 and 90.8% of modified AASHO maximum, respectively, both at the 0- to 2-in. depth. As shown in plate 6, the densities produced by compactor B were much lower than those of the rubber-tired roller for all three moisture contents. Compactor B produced from 95% of the compaction of the rubber-tired roller at dry of optimum moisture content to 100% at wet of optimum at the 0- to 2-in. depth, but only about 87% of the compaction of the rubber-tired roller at the ϵ - to 8-in. depth for all three moisture contents. The density-coverage data of table 4 and plate 9 show increases between 4 and 16 coverages of 7.4, 8.8, and 3.0 pcf for the dry of optimum, optimum, and wet of optimum moisture contents, respectively. Maximum compaction after 4 coverages occurred at wet of optimum moisture, and maximum compaction after 16 coverages occurred at optimum moisture.

Compactor C

Sand test section

15. Compactor C, a self-propelled vibratory compactor with a gross weight of 5270 lb, achieved the highest densities of the vibratory compactors in the sand. The maximum values obtained were 95.3, 97.2, and 102.5% of modified AASHO maximum (all at a depth of 6 to 12 in.) in the dry of optimum, optimum, and wet of optimum moisture items, respectively. Plate 4 shows that between the 0- and 12-in. depths, compactor C produced densities approximately equivalent to those of the rubber-tired roller at dry of optimum and optimum moisture contents, and as much as 111% of the

compaction of the rubber-tired roller at wet of optimum moisture at this same depth. For depths greater than 9 in., compactor C produced about 95% of the compaction of the rubber-tired roller. The surface density-coverage data were similar to those of the other vibratory compactors with a decrease in density between 4 and 8 coverages and then an increase to a maximum value at 16 coverages. At 8 coverages, the densities of the rubber-tired roller were higher than those of compactor C, but after 16 coverages, compactor C densities were equivalent to those of the rubber-tired roller at optimum moisture content and higher at the other two moisture contents. The rubber-tired roller showed an increase in density at optimum moisture content and a decrease in density at dry of optimum and wet of optimum between 8 and 16 coverages.

Limestone test section

AASHO maximum at a depth of 0 to 6 in. and optimum moisture content, with the highest densities at dry of optimum and wet of optimum being 92.7 and 94.8% of modified AASHO maximum, respectively, both at the 0- to 6-in. depth. Compactor C achieved the following percentages of the compaction of the rubber-tired roller: 99% in the dry of optimum moisture item (average of values for all depths), 96.3% at the 0- to 6-in. depth and 97.0% at the 6- to 12-in. depth for the optimum moisture item, and 96.1% at the 0- to 6-in. depth and 89.8% at the 6- to 12-in. depth for the wet of optimum item. At the dry of optimum and wet of optimum moisture contents, compactor C densities showed an increase of about 3 pcf between 4 and 16 coverages. At optimum moisture, there was a decrease in density between 4 and 8 coverages and then an increase to 16 coverages with the increase between 4 and 16 coverages being 6.4 pcf.

Lean clay test section

17. Compactor C achieved a maximum density of 91.7% of modified AASHO maximum at a depth of 0 to 2 in. in the lean clay at the wet of optimum moisture content. The maximum density values for the dry of optimum and optimum moisture contents were 87.1 and 91.3% of modified AASHO maximum, respectively, both at the 0- to 2-in. depth. As shown in plate 6, compactor C's density values are appreciably lower than those of the

rubber-tired roller except at the 0- to 2-in. depth in the wet of optimum moisture item, where they are about equal. The density increase for the rubber-tired roller between 4 and 16 coverages was 3.7 pcf for the dry of optimum moisture item, but only 1.4 and 1.0 pcf for the optimum and wet of optimum moisture items, respectively. Compactor C showed density increases between 4 and 16 coverages of 9.0 pcf in the dry of optimum item, 10.0 pcf in the optimum item, and 7.7 pcf in the wet of optimum item.

PART IV: SUMMARY OF RESULTS, AND CONCLUSIONS

Summary of Results

18. The overall compaction ability (with the materials and moisture contents used in these tests) of the compactors used in this investigation is tabulated below. The compactors are rated numerically in order of effectiveness (1 is best, 2 second best, etc.).

		Nume	erica	l Rati	ing of	Cor	mpact	tors A	, B,	С, ε	and D)
	Dr		Opti			-	mum		Wet		Opti	
		Mois	sture	;	1	loiಟ1	ture			Moi:	sture	;
<u>Material</u>	A	В	C	D	A	В	C	D	A	В	<u>C</u>	D
Sand	3	4	2	1	4	3	2	1	4	3	1	2
Limestone	1	4	3	2	1	3	4	2	1	3	4	2
Lean clay	2	4	3	1	2	4	3	l	2	4	3	1

- 19. The information obtained from this investigation warrants the following observations:
 - a. Compactor A, the heavy, low-frequency compactor, produced compaction similar to that of the 50-ton rubber-tired roller. Compactor A gave good compaction in limestone and lean clay, but not in sand.
 - b. Compactor B, the lightweight, high-frequency compactor, showed good performance in the sand only, and this was at optimum and wet of optimum moisture and for a depth of 0 to 12 in. For most other materials and conditions tested, compactor B gave the poorest compaction.
 - c. Compactor C, which was of intermediate weight and low frequency, produced the best compaction of any vibratory roller in the sand. It produced over 100% of modified AASHO maximum density at the 0- to 12-in. depth at wet of optimum moisture. Below the 0- to 12-in. depth, density fell off rapidly; therefore, this compactor would not be effective for lifts thicker than approximately 9 in. Compactor C was much less effective for compaction of limestone and lean clay than was compactor A or the rubber-tired roller.
 - d. In the lean clay, about 4 coverages of the 50-ton rubbertired roller were equivalent to 16 coverages of compactor A (the most effective vibratory roller in the lean clay).

- e. The vibratory compactors gave their best compaction at wet of optimum moisture in the sand and at optimum moisture in the limestone and lean clay.
- 20. No analysis was made of the effect of frequency or centrifugal force of the vibratory compactors because sufficient information was not available to determine the force actually applied to the soil.

Conclusions

- 21. There is evidence that in sand being compacted with vibratory rollers the density will be a cyclic function of coverages. Plate 7 shows higher densities for 4 and 16 coverages than for 8 coverages. The effect of frequency is most apparent in sand. The lowest frequency middleweight compactor (C) did the best overall job of compacting the sand. For the limestone and lean clay it appears that the deadweight of the roller was the most important factor. Densities in these materials generally increased in direct proportion to increases in the deadweight of the vibratory rollers.
- 22. The results of the tests show that vibratory rollers, in spite of the claims of manufacturers, will not produce densities to any significantly greater depth than will rubber-tired rollers (exception: compactor A in limestone).
- 23. For comparable lift thickness of compaction, it may be possible to substitute much lighter vibratory rollers for heavy rubber-tired rollers; however, there is a limit to the amount of weight reduction that can be achieved through use of vibratory rollers.

Tat'le 1

Lurrary of Density and Moisture Content Data, Sand Test Section - 16 Coverages

	% Nodifie: AASHO Max		7. 7. 7. 8. 8. 8. 7. 7. 6. 7.		22.1 20.3 100.1 100.1 77.4 77.3		4900013 100013 100013	
	Compactor D t Dry Density pcf		100.3 110.1 100.1 105.9 100.3 100.1		4.501 4.501 4.501 4.501 6.111 6.111		104.7 114.3 114.3 114.1 1058.1 105.6	
	Co Mcisture Content & Dry Wt		6.33 C				7 m. v m m m m 1 1 1 2.	
and and and	& Moitried AASHO Max		2000 20 20 20 20 20 20 20 20 20 20 20 20		नेहर्रहें ।।। देह		4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4	
A00 07 - 1010	Compactor C it Dry Pensity Fei		107.1. 105.8 105.8 105.7		10.5			
	Moisture Content Moisture Content	Dry of Optimum Moisture (7.2%)	2000 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	(31.0-12.0)		stran (1. 1)	1. 4. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	
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	Compactir F t Dry Density Fef	Dry	5000 5000 5000 5000 5000 5000 5000 500	rtim		Jo + 3%		
	Moisture Conten § Dry Wt		1 1 1 4 0 0 0 0 0 1 1 1 0 0 0 0 0 0 0 0		5.7.7.7.7.7.7.7.7.7.7.7.7.7.7.7.7.7.7.7			
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Summary of Density and Moisture Content Data, Limeatone Test Section - 16 Coverages

	Avg % Modified AASHO Max		93.3		94.6		85.9
or D	<u> </u>		141.2 140.7 136.5 138.6		149.4 149.0 141.0 14.8		147.8 143.3 143.3
Compactor D			ਜਿੰਦੇਸ		ลลลล		2222
	Moisture Content % Dry Wt		म्पूम्म इंट्यं <i>जं</i>		ับบุญ ว่าข้า		യയയയ ഇപ്പോൾ
	ASHO Max		92.7		95.6 8.8		8.46
Compactor C	1 3		142.0 138.2 132.5 138.9		143.9 145.0 136.6 132.0		142.0 144.7 131.7 134.1
Con	Moisture Content \$ Dry Wt	Dry of Optimum Noisture (1.0%)	नम्बन न्युव्य	re (5.0%)	ดดษด คัญจั๋	Wet of Optimum Moisture (7.0%)	0.0.0.0 6.6.4.0.
	Avg Modified	Optimum Noi	91.6	Optimum Moisture (5.0%)	92.5	f Optimum Mo	%:- 92:9
Commeter B	Dry Density pef	Dry of	138.5 135.3 136.3	췽	143.8 142.5 140.7 138.9	Weto	146.5 143.9 139.3 141.5
	Moisture Conter \$ Dry Wt		HOHH Om ww		4.11.14 4.0.00		चल जब छ नं ं ं
	% Modified		92.5		98.1 95.8		97.9
A word of the	Dry Denaity		141.3 138.4 139.9		149.1 147.4 140.3 148.7		148.7 148.2 148.2 147.3
2	Moisture Content		4004 4004		4.6.5.4 6.4.6.4		യം വേഗ ഗ ഗ്യൂർം
	Depth in.		0-6 0-6 6-12 6-12		0-6 0-6 6-12 6-12		004 6-12 6-12

Table 3 Jurrary of Density and Moisture Content Data, Lean Clay Test Section - 16 Coverages

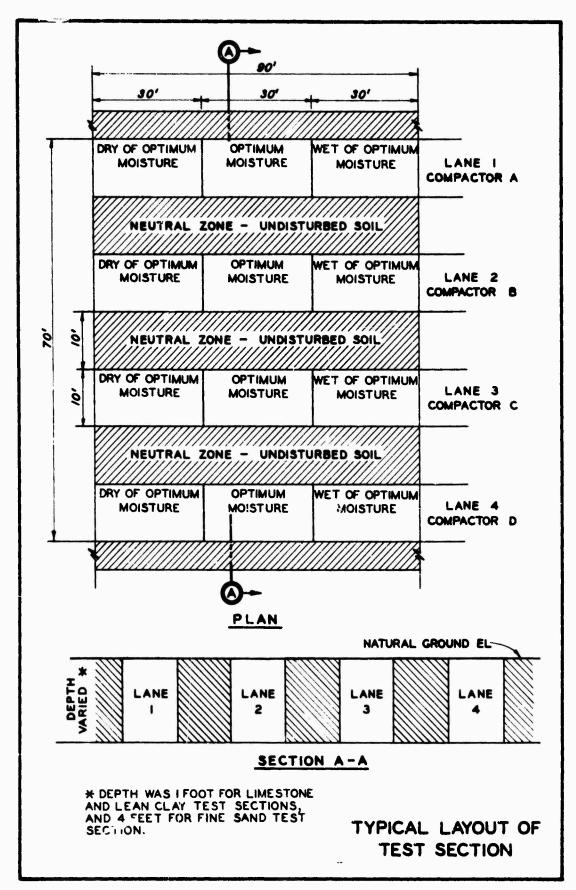
	ASHO Nax		31.5 87	0 G.	6.1 1	: :	:::		7 67	- d	€-1-8 -1-8	'ဆွဂ် 'ဆွဂ်	? 0	1 :	1	! :			\$ 2.5	30	 2 S	1	1	: : :	1	
Compactor D	Dry Pensity per		105.0	97.6 95.4	95.8 10.2.4	10 6. 3.	22.87.24 4.11.10.10		0 00	107.6	LOJ E E COJ	1001	110.7	108.1	103.2	i. i. •		4.1	: 1	13	* * * * * * * * * * * * * * * * * * *	2.7ct	:	107.6	1.161	
2	Moisture Content % Dry Wt		13.7	12.0	ं है। हैं हैं।	~ 0.6 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4	8.9.0. 1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.		200	15.	18.4	17.4	16 °1.	18.0	17.0	િ.સ •		0	2.00	5 C	역 - 1 위 (1	13.	5,7	7	: :	
	Av. % Modified AASHQ Max		87.1 8.1.	7.77	: :	::	:::		0 0	. ુ નુકુ	യൂം സ. ഗ്ര ജാർ	76.1	: :	:	: :	: :		:	a) 1	₽Å vo	77.7	:	: :	::	• •	
Compactor C	Dry Density Fof		101.0 96.6 91.0	7.00	102.5	97.7 91.2	91.3		-	**************************************	9 7. 8	: m : d	107.5	103	۶.غ م	11.7		0	1 (S)	37.5	73.7	\$	φ	٠. ١ . ٠	:	
CO	Moisture Content & Dry Wt	Dry of Optimum Moisture (13.0%)	માં માં જ અ વ્ય) स्टा	ဖွာ့ (၈)	13.7	ನ್∾ ೯ನೆ ಕರೆ	e (14.7%)		- e-67	If on	17.9	1 2 2	13.8		1.11	Wet of Ptimum Moisture (5 .€)		10 10 10 10 10 10 10 10 10 10 10 10 10 1	15.0 - 0.0	1.0.	•		7°C	:	
	Avg % Modifiet AASHO Max	Optimum Mois	88.6 43.03	5.	::	::	111	Optimum Moisture (14.%)		• ' ક દ્ર* ફ.	Ĉ,	<u>}</u>	: :	l :	: :	: :	ptimm Mei	:	1. id 2. gi		71	:	1 1	::	;	
Compactor B	Dry Density pef	Dr.y of	101.2	57.7	 	7.17 21.7	T : :	iado	ļ	σ n. τ	۳.	- -	1	1,0		: :	Je de	,	 4 -	i	2.	بر بر بر	. '.	σ.Α. ed ;	:	
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Č	District Content Dry Seatty		् लन्		, 1]	1	2.1			77	• • •	<i>()</i>	i :		4 4	<i>:</i> 1	l			•] •	1.		• •	• •	1	
			<u>,</u> i)] [i	- 7				į	<u>.</u> '			ì !	7	ì			١.	i.		• •			

Tatle 2. of Density, M.isture, and Coverage Data at

			Remarks		Direct	Direct	Direct		Road logger	Road logger	Road logger	Direct		Direct	Direct
	i de	Dry Den-	DCI		*	110.2	104.7		139.6	141.3	143.7	145.6		106.0	107.0
	Wet of Optimum	Mois- ture Content	1		*	9.1	6.6		8.5	8.8	8.6	3.3		20.5	19.8
tor D	371	Den-	I Sci			6.98	105.4		136.0	135.4	137.0	149.2		108.4	109.8
Compactor D	Octu	Mcis- Dr tur De Content si	1		•	7.8	6.9		7.5	7.2	7.4	٧. د.		18.0	16.8
	Jo Britis	Pro fty	ğ		999.5	100.4	100.3		13.8	134.1	136.1	1,1,1		103.5	107.
	Dry of Optimum	Mois- Dr ture De Content si	1		€.€	5.0	6.3		5.6	9:,	9.3	1.2		14.7	12.2
	of mum		Joe L		105.8	103.4	116.3		136.9	137.8	140.2	143.3		99.8	107.5
	Wet of Ortimum	Mois- ture Content	1		o. 2	°."	89		8.1	7.8	-1 CC	2.7		6.15	19.3
tor C	alum mina		Joa .		101.2	95.0	6.401		135.7	134.5	142.1	144.5		31.0	107.0
Compactor C	Optimum	Mois- ture Content	4		9.3	8.1	6.1		6.6	8.3	6.5	5,€		17.6	17.8
	of mum		j od	5	100.6	1001	0.701	tion	137.	138.1	140.2	140.2	tlon	93.1	1.501
	Dry of Optimum	Mois- ture Content	4	st Secti	8.	6.3	₹.4	est Se	J. 22	2.7	3.0	1.3	lest Sec	13.8	13.5
	り		101	Sanl Test Section	106.3	103.9	113.c	Limestone Test Section	137.8	136.4	137	145.3	Lean Clay Test Section	103.4 13.8	104.
	Wet of Outlinin	Nois- ture Content	4		9.9	9:21	7.3	ᆌ	7.8	7.7	0.9	0.	3	1.02	30.5
tor B	mn		101		104.5	¥.3	4 · · · · · · · · · · · · · · · · · · ·		1.35.1	135.5	135.4	143.2		8	107.4
Compartor B	Optinum	Mois- ture Content				6.9	5.5		5.4	5.1	5.t	1.5		17.3	0.71
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Table 5 Ground Deflection Data

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est Sect	1- Ootimum Wei	Moisture	Dis-	+400	In.			0.013	0.0068	0.017	1000	1	}	;	0.029	0.028	440.0	0.045	000	1000	1000				0.005	:	0.003	:	0.008)))	ָרָ הַ בַּי		200	7		2	3 6	0.010	200	0.00		
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	Mot of Onti-	mum Moisture	Dis-	Prace.	1n.			0.0525	0.0346	0,00		2000	2000	0000	0.0680	0.0739	0.0736	0.0756		71.10	tt.0.0			1-ft Depth	0.00	0.0132	0.0139	0000		6000	0.0000	0.0104	0.0100	:		10.00	0.0154	0.0620	0.0237	0.0 0.0 0.0 0.0 0.0	0.0	
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Limestone Test Section	Ont this	Moisture	D3 2-	in ace	in		Compactor A, 1-ft Depth	4110.0	0.0300		0.0262	0000		0.0533	0.0375	0.0644	0.0684	0.0766	2000	2000	0.0039	B			18 to 0	0.0112	0.0075	000	4410	00000	20.00	0.0455	0.0038	:	O	1	0.0254	0.0297	0,0400	0.03Tc	30.0	
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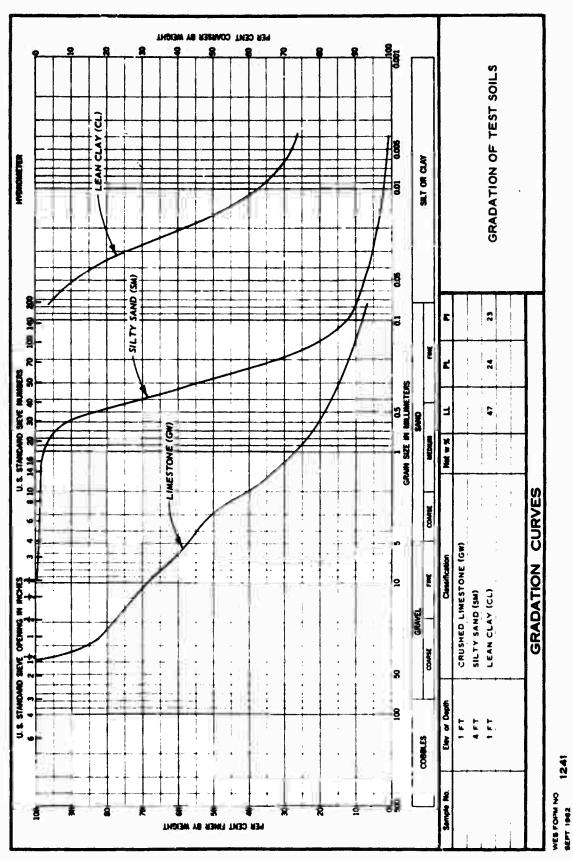


PLATE 2

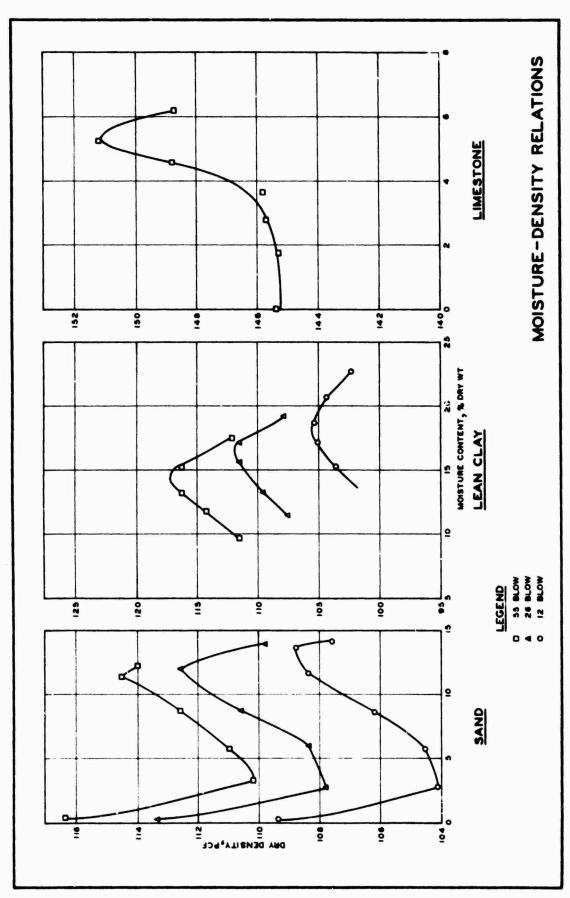


PLATE 3

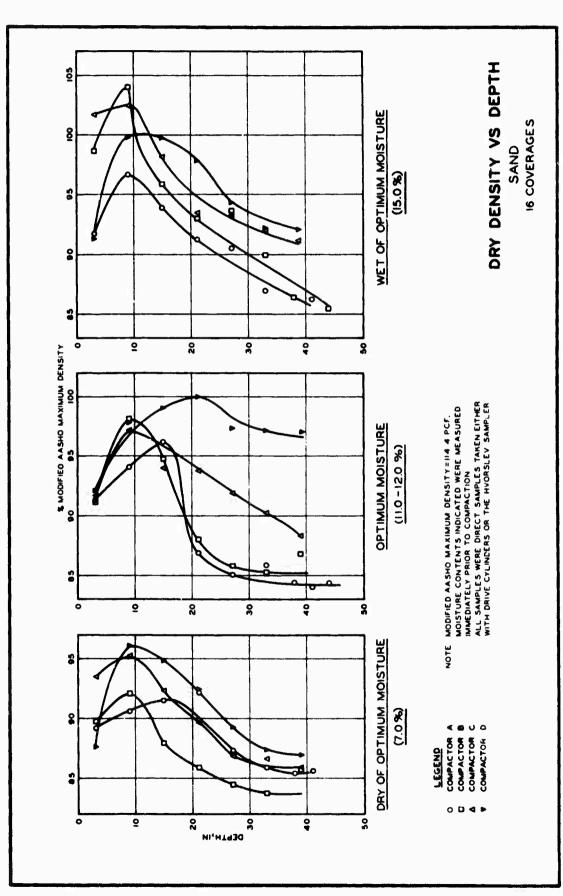
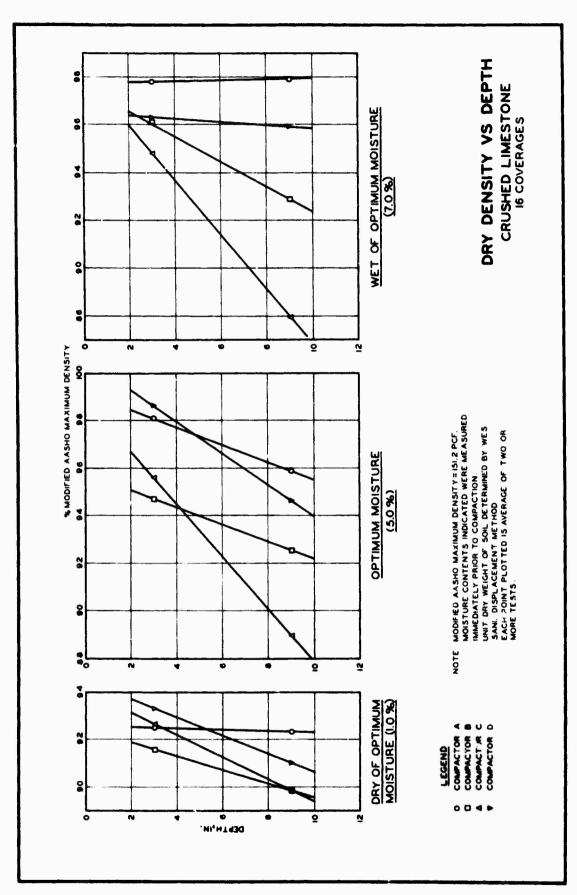
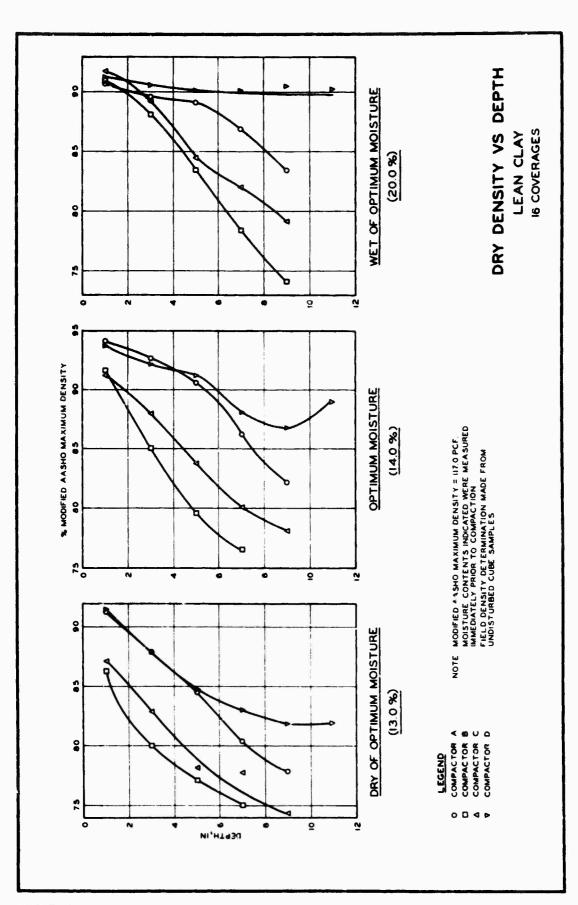


PLATE 4





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PLATE 6

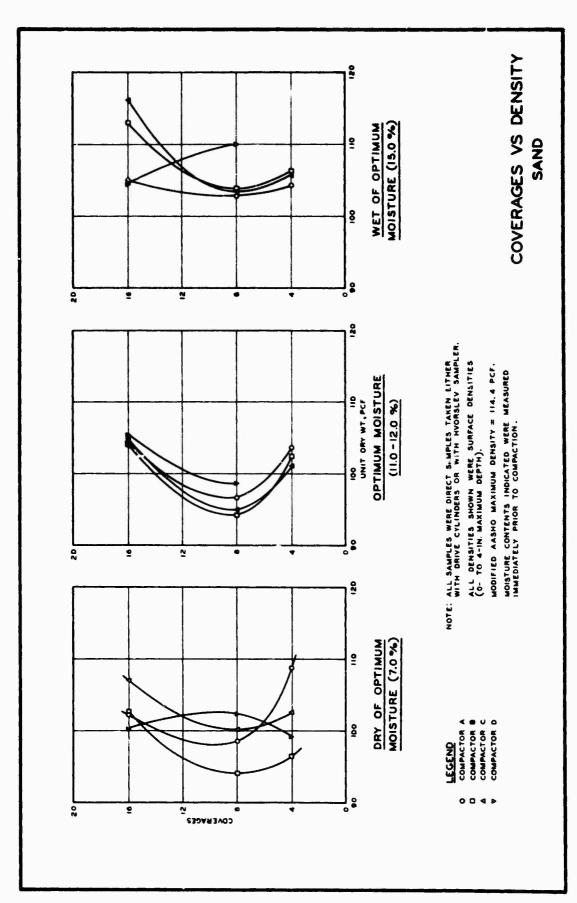


PLATE 7

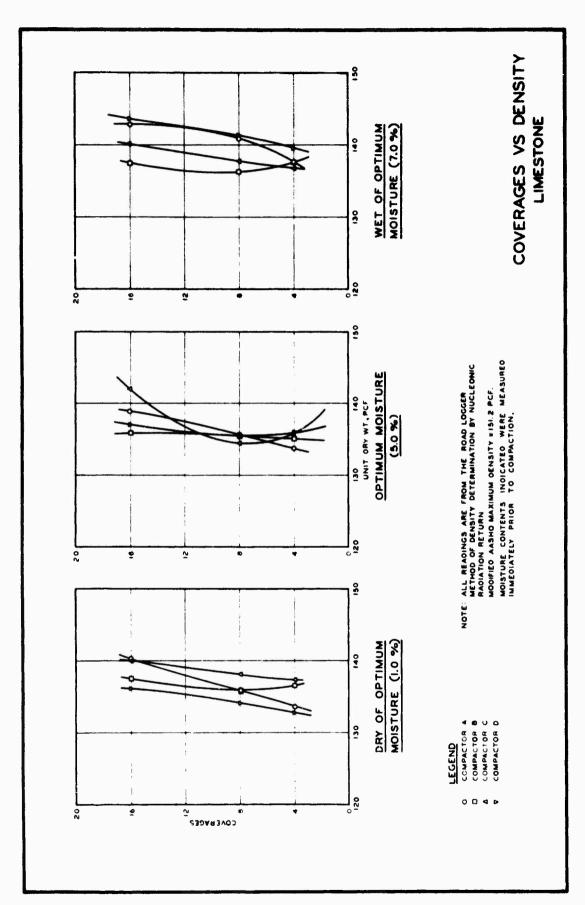


PLATE 8

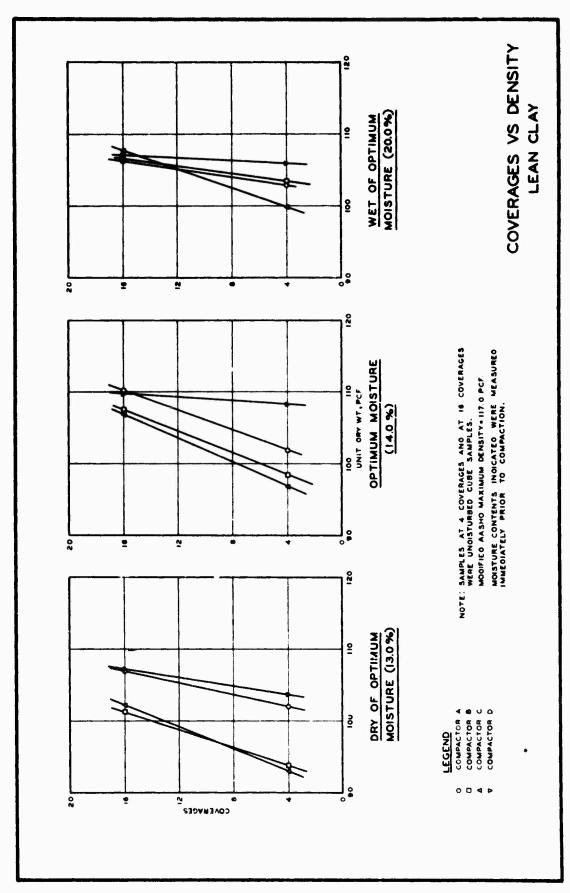


PLATE 9

Unclassified Security Classification			
DOCUMENT CONT			
(Security classification of title, body of obstract and indexing a 1. ORIGINATING ACTIVITY (Corporate outlier) U. S. Army Engineer Waterways Experiment St Vicksburg, Mississippi			CURITY CLASSIFICATION
SOIL COMPACTION INVESTIGATION; EVALUATION OF SOILS	F VIBRATORY	ROLLERS ON	N THREE TYPES
e. OESCRIPTIVE NOTES (Type of report and inclusive dates) Report 10 of a series S. AUTHORISI (First name, atddis initial, last name)			
Jim W. Hall			
March 1968	74. TOTAL NO. 0	FPAGES	76. NO. OF REFS
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This document has been approved for public unlimited.	release and	sale; its	distribution is
11. SUPPLEMENTARY NOTES	U. S. Ai:		VIVV
This study was conducted for the purpose of rollers to compact soils. For comparative used which is a required compaction device cations. Three vibratory rollers were sele frequency which encompassed the range over Results of this study show that light vibrat densities if lift thicknesses are restricted each was used to compact three soil types (clean sand). Results indicate that a heavy pact to greater depths than a light, high-if frequency roller will compact soil satisfact Soil types have a very definite influence of the vibratory rollers generally perform betom-frequency type rollers do a satisfactor.	purposes, a in present (sected for structured for structured). To evaluate the control of the co	50-ton rul Corps of Ex udy based on t vibrators s can obtain that the violation , a crushed ency vibrate ller; howers a few inches btained with ular soils	bber-tired roller was ngineers Guide Specifi- on their operating ry rollers operate. in satisfactory ibratory rollers, d limestone, and a tory roller will com- ver, the light, high- es below the surface. th vibratory rollers. ; however, the heavy.

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